## Lake Ripley Watershed 75% Geotechnical Report

Requested by: Lake Ripley Management District in Cambridge, Wisconsin

## **OCTOBER 21<sup>ST</sup>, 2021**

**BEAVER PROFESSIONAL SERVICES** Authored by: Faith Gillen, Will Mockert, Luke Senta, & Caroline Skotarzak





NATURE BASED SOLUTIONS



**Beaver Professional Services** 324 Wendt Commons 215 N. Randall Avenue



Ms. Lianna Spencer Lake Manager N4450 County Road A Cambridge, WI 53523

Madison WI 53715

October 21<sup>st</sup>, 2021

RE: Subsurface Soil Investigation Report Lake Ripley Management District Preserve Village of Cambridge Cambridge, WI

Dear Lianna Spencer,

Per your request, Beaver Professional Services has performed a Geotechnical Engineering Subsurface Investigation and Report for the Engineering Services for Watershed Improvements project. Enclosed you will find our "75% Geotechnical Report – Lake Ripley Management District Preserve," which showcases our conclusions and recommendations.

If there are any questions, comments, or concerns that arise as you review this report or if our firm can be of further service during the construction phase of the project, please feel free to reach out to me or any member of our team. Our goal is to make this project as seamless and successful as possible for the Lake Ripley Management District.

Thank you for your time,

aroline Rfkotanjak

Caroline R. Skotarzak Project Manager Beaver Professional Services

Enclosed:

Report (pg. 1) Location Map (pg. 8) Boring Logs (pg. 20)



## **DISCLAIMER: Student Project Documents**

The concepts, drawings and written materials provided here were prepared by students in the Department of Civil & Environmental Engineering at the University of Wisconsin-Madison as an activity in the course Civ Engr 578 – Senior Capstone Design/GLE 479 – Geological Engineering Design. These do not represent the work products of licensed Professional Engineers. These are not for construction purposes.



## Introduction

Beaver Professional Services has completed geotechnical investigation for the area of proposed watershed improvements surrounding Lake Ripley in Cambridge, WI. This investigation focuses on subsurface exploration adjacent to the Lake Ripley influent stream which will be the location of design. It also factors in locations on the Lake District Preserve just north of the influent stream that may be impacted by proposed designs.

This document contains analysis of soil and geological conditions of the project site. Recommendations will be made based on this investigation to limit the impact that site conditions could have on construction of engineering design alternatives.

## **Project Description**

The Lake Ripley Management District seeks to improve water quality in Lake Ripley by making improvements to the lake's influent stream. This stream travels through the Lake District Preserve in addition to neighboring properties. Three proposed alternatives aim to redirect the stream to meander through the Lake District Preserve where native prairie species will strip the surface water of pollutants such as phosphorus as well as settling out suspended solids in the water before it reaches the lake.

The project area lies in a generally flat prairie wetland at an approximate elevation of 842 ft. There are no existing structures on the preserve, and there is a hill to the east of the proposed stream redirection that peaks at an approximate elevation of 884 ft.

## Scope of Subsurface Exploration

Hollow stem augers were used to perform standard penetration test borings of the project area. Five soil borings (Borings 1-5) were collected for testing. All collected soil boring samples are included in Figure 9-13. Three were taken directly adjacent to the influent stream, and two were taken in the flat wetland north of the stream where redirected surface water will flow towards Lake Ripley. Boring 1 was taken at a location on the west side of the Lake District Preserve where a weir is proposed for Alternative 3. A detailed map of the boring locations can be found in Figure 3-4. Soil borings were drilled to a depth of 30 ft. This depth was sufficient to explore the existing soil conditions, but drilling did not need to extend to further depths due to the absence of any significant infrastructure being constructed on the site location.

Selection of boring location was determined based on proximity to where construction will take place in the three proposed alternatives. A boring was taken at the location where a weir would be implemented. The installation of a weir would require the presence of machinery and equipment that would affect soil conditions, so it is important to consider whether the presence of this equipment



would cause erosion of the streambanks. Another two borings were taken near the stream on the northside of where a berm and diversion would be implemented or the beginning of new stream excavation to gauge how the water table changes moving away from the influent stream. The final two borings were taken in the wetland to gain an understanding of the prairie's soil condition. Should a new channel be excavated through the prairie, machinery would need to enter this area. No matter what alternative is selected, the goal is to redirect water from the influent stream towards the wetland to then flow towards Lake Ripley. The team will need a comprehensive analysis of how increased surface water flow will affect soil conditions in this area.

To collect soil boring samples, a standard penetration test was performed at each location using hollow stem augers. This process was completed in accordance with ASTM D1586. This testing procedure obtains samples at discreet depths and provides a standardized blow count that can be used to analyze a soil sample. Collection and testing were performed by Beaver Professional Services geotechnical engineering team on October 1, 2021.

## Site Description

## Area Geology

Lake Ripley is located in Cambridge, WI in Jefferson County. An estimation of existing area geology was collected from the Natural Resources Conservation Service (NRCS). A map of the NRCS data for the Lake Ripley project location is illustrated in Figure 5. Approximately 50 percent of the intended site location consists of Houghton muck. A detailed description of Houghton muck is located in the "NRCS Soil Survey Information" in the Appendix. This section of the project site is the low-lying prairie in the Lake District Preserve. Houghton muck is abundant in herbaceous organic material, and in the site location of this project, it is very poorly drained. Houghton muck is accompanied in the project area by small sections of Wacousta silty clay loam and Casco-Rodman complex soil. These sections are located on the eastern hill and on areas of higher elevation on the project site. Detailed descriptions of both soil types are also located in the "NRCS Soil Survey Information" in the Appendix.

## Subsurface Conditions

Collected soil borings show a distribution of a sand and clay layer to approximate depths of 5 ft where it then transitions to lean clay layers up to an approximate 15 ft depth. Moving deeper in the boring, a sandy layer with gravel is located from approximate depths of 15 ft to 27 ft. Finally in the deepest section of the soil boring, a very dense layer is found that shows characteristics of weathered sandstone bedrock. A schematic of the general soil cross-section for the Lake Ripley project site is laid out in Figure 1, further details of each individual boring can be found in Figures 9-13 in the Appendix.



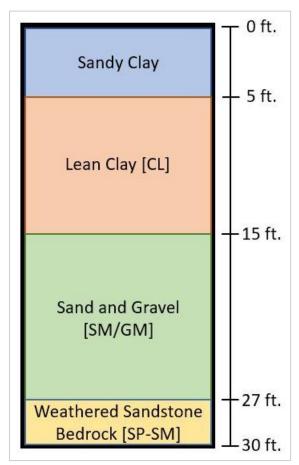


Figure 1: General Soil Cross-Section on Lake Ripley Preserve

## **Groundwater Conditions**

Water table elevation plays an important role in borings collected from the Lake Ripley project site. Groundwater was encountered at 2-3 ft. in the soil borings adjacent to the influent stream and at 5 ft. depth for the borings in the central area of the Lake District Preserve. Groundwater levels found closer to the surface is a result of the proximity of the Lake Ripley influent stream to the boring location. Groundwater conditions will continue to be a factor in engineering design of this project because it will affect the construction conditions of a proposed alternative. Settlement caused by excavators on the site will need to be determined to avoid unnecessary delays caused by wet conditions in the surrounding area.

## **Environmental Concerns**

While the collected soil borings do not show signs of contamination, it is recommended that a more detailed environmental investigation and report be conducted prior to the beginning construction services for this project.

## **Discussion and Recommendations**



## Site Selection

When investigating the differences between the location of where a weir would be installed versus the location where a berm would be implemented of a new channel excavated, there are minimal differences because both sites are directly adjacent to the influent stream. There is a general uniformity among the characteristics of all soil borings that shows there is no need to limit site selection based on geotechnical advantage.

## **Bearing Capacity**

The footprint of the 30,000 lb excavator to be used in construction is 100 sq. ft. This corresponds to a 300 psf loading from the excavator that must be resisted by soil during construction. Allowable bearing capacity calculations found in the Calculations section of this report indicate that the project site has a bearing capacity of 3310 psf with a factor of safety of 3.0. For this reason, excavation does not pose a reason for concern over geological site conditions.

## Geotechnical-Related Construction Issues

## Season of Construction Considerations

The bulk of work will take place in the upper layer of sandy clay topsoil, creating a potentially unstable environment for larger equipment. Due to these unstable surface conditions, BPS is recommending all construction activities requiring heavy equipment to be executed during the Winter months, when the ground is frozen, so that soil conditions are at their most stable. If using heavy machinery when the soil is thawed, BPS recommends using a swamp mat or another wetland construction method to ensure safe use of equipment.

## Soil Reuse

Soil excavated In each of the three alternatives will be used to grade the streambank on the north side where overflow is expected in each alternative. In Alternative 1, there will be less need for soil reuse and more need to dispose of excavated soil. Any excess soil that cannot be reused on the project site will be transported by dump truck to a landfill for a cost of \$0.025 per pound of soil.

## Calculations

## Ultimate Bearing Capacity ( $Q_u$ ):

For soil type found in NRCS data, attributes estimated include:

- Cohesion, c = 1.5 psi
- Friction angle,  $\Phi'$ =30 degrees, therefore:
  - $\circ N_a = 22.46$
  - $\circ N_c = 37.16$
  - $\circ N_{\gamma} = 19.70$
- Saturated unit weight of soil, 100 pcf



- Unit weight of water, 62.4 pcf
- D = depth of footing = 0.5 ft. for 6" weir installation
- B = footing width = 4 ft.

$$Q_u = cN_c + \gamma DN_q + 0.5\gamma BN_{\gamma}$$

$$Q_u = 1.5psi * \frac{144in^2}{ft^2} * 37.16 + (100pcf - 62.4pcf)$$

$$* 0.5 ft * 22.46 + 0.5$$

$$* (100pcf - 62.4pcf) * 4ft * 19.70$$

$$= 9930.25 psf$$

## Allowable Bearing Capacity (**Q**<sub>all</sub>):

Factor of Safety (FS) = 3, therefore:

$$Q_{all} = \frac{Q_u}{FS} = (9930.25 \, psf)/3.0 = 3310.08 \, psf$$

## Appendix

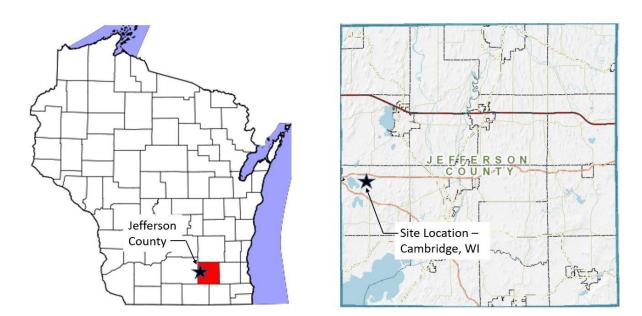


Figure 2: Project Site Location



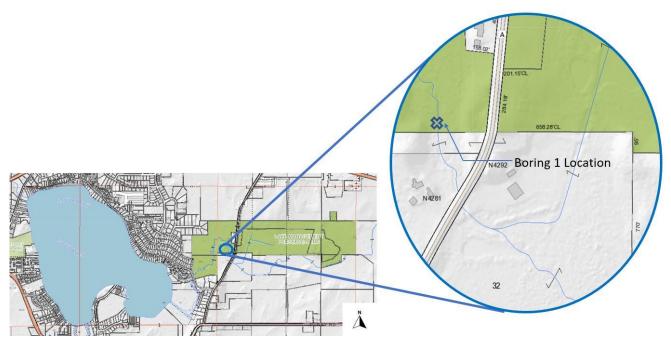


Figure 3: Soil Boring 1 Location

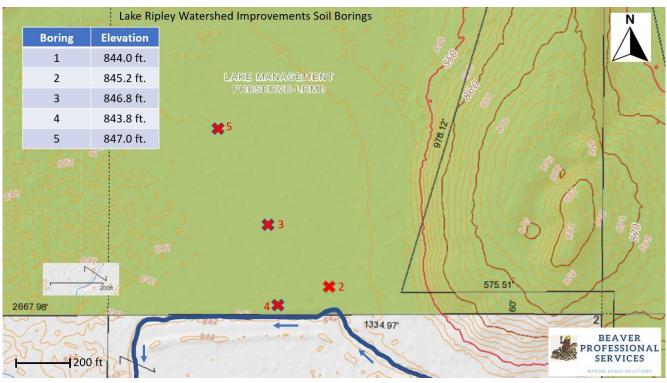


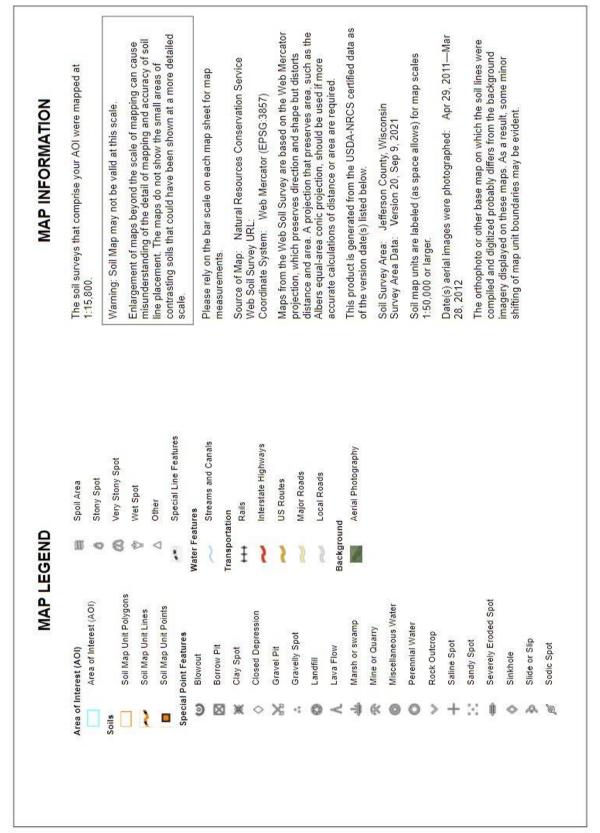
Figure 4: Soil Borings 2-4 Location



Figure 5: NRCS Map



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Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
CrD2	Casco-Rodman complex, 12 to 20 percent slopes, eroded	.14.1	15.1%
FoC2	Fox loam, 6 to 12 percent slopes, eroded	6.9	7.4%
FsB	Fox silt loam, 2 to 6 percent slopes	6.0	6.4%
Ht	Houghton muck, 0 to 2 percent slopes	47.6	51.0%
RaA	Radford silt loam, 0 to 3 percent slopes	1.1	×1.2%
VwA	Virgil silt loam, gravelly substratum, 0 to 3 percent slopes	4.4	4.7%
Wa	Wacousta silty clay loam, 0 to 2 percent slopes	13.2	14.1%
Totals for Area of Interest		93.3	100.0%

## Map Unit Legend





Map Unit Description: Houghton muck, 0 to 2 percent slopes---Jefferson County, Wisconsin

#### Jefferson County, Wisconsin

#### Ht—Houghton muck, 0 to 2 percent slopes

#### Map Unit Setting

National map unit symbol: 2szff Elevation: 600 to 1,090 feet Mean annual precipitation: 31 to 35 inches Mean annual air temperature: 43 to 48 degrees F Frost-free period: 124 to 192 days Farmland classification: Farmland of statewide importance

#### Map Unit Composition

Houghton, muck, and similar soils: 90 percent Minor components: 10 percent Estimates are based on observations, descriptions, and transects of the mapunit.

#### Description of Houghton, Muck

#### Setting

Landform: Depressions Landform position (three-dimensional): Dip Down-slope shape: Concave Across-slope shape: Concave Parent material: Herbaceous organic material

#### Typical profile

Oap - 0 to 6 inches: muck Oa - 6 to 79 inches: muck

#### Properties and qualities

Slope: 0 to 2 percent Depth to restrictive feature: More than 80 inches Drainage class: Very poorly drained Runoff class: Negligible Capacity of the most limiting layer to transmit water (Ksat): Moderately low to high (0.14 to 5.95 in/hr) Depth to water table: About 0 to 4 inches Frequency of flooding: None Frequency of ponding: Frequent Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm) Available water supply, 0 to 60 inches: Very high (about 23.9 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 3w Hydrologic Soil Group: A/D Hydric soil rating: Yes



Minor Components

#### Houghton, ponded

Percent of map unit: 4 percent Landform: Depressions Landform position (three-dimensional): Dip Down-slope shape: Concave Across-slope shape: Concave Hydric soil rating: Yes

#### Palms

Percent of map unit: 2 percent Landform: Lakebeds (relict) Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Base slope Down-slope shape: Concave Across-slope shape: Concave Hydric soil rating: Yes

#### Adrian

Percent of map unit: 2 percent Landform: Lakebeds (relict) Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Base slope Down-slope shape: Concave Across-slope shape: Concave Hydric soil rating: Yes

#### Willette, muck

Percent of map unit: 1 percent Landform: Depressions Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Base slope Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: Yes

#### Edwards

Percent of map unit: 1 percent Landform: Depressions Landform position (three-dimensional): Dip Down-slope shape: Concave Across-slope shape: Concave Hydric soil rating: Yes

#### Data Source Information

Soil Survey Area: Jefferson County, Wisconsin Survey Area Data: Version 20, Sep 9, 2021

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Natural Resources Conservation Service

#### Web Soil Survey tional Cooperative Soil Survey

10/5/2021 Page 2 of 2



Map Unit Description: Casco-Rodman complex, 12 to 20 percent slopes, eroded---Jefferson County, Wisconsin Casco-Rodman complex

#### Jefferson County, Wisconsin

#### CrD2—Casco-Rodman complex, 12 to 20 percent slopes, eroded

#### Map Unit Setting

National map unit symbol: 2tjwm Elevation: 640 to 1,200 feet Mean annual precipitation: 29 to 35 inches Mean annual air temperature: 43 to 48 degrees F Frost-free period: 124 to 192 days Farmland classification: Not prime farmland

#### Map Unit Composition

Casco, eroded, and similar soils: 53 percent Rodman, eroded, and similar soils: 37 percent Minor components: 10 percent Estimates are based on observations, descriptions, and transects of the mapunit.

#### Description of Casco, Eroded

#### Setting

Landform: Moraines Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope Down-slope shape: Linear Across-slope shape: Convex Parent material: Loamy alluvium over calcareous, stratified sandy and gravelly outwash

#### **Typical profile**

Ap - 0 to 5 inches: loam Bt - 5 to 17 inches: clay loam 2C - 17 to 79 inches: stratified sand to gravel

#### Properties and qualities

Slope: 12 to 20 percent
Depth to restrictive feature: 11 to 20 inches to strongly contrasting textural stratification
Drainage class: Somewhat excessively drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.60 to 2.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum content: 25 percent
Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Available water supply, 0 to 60 inches: Very low (about 2.8 inches)

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Web Soil Survey

10/5/2021 Page 1 of 3



#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 6e Hydrologic Soil Group: B Forage suitability group: Low AWC, adequately drained with limitations (G095BY003WI) Other vegetative classification: Low AWC, adequately drained with limitations (G095BY003WI) Hydric soil rating: No

#### Description of Rodman, Eroded

#### Setting

Landform: Moraines Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope Down-slope shape: Linear Across-slope shape: Convex Parent material: Sandy and gravelly outwash

#### Typical profile

Ap - 0 to 4 inches: gravelly sandy loam Bw - 4 to 10 inches: gravelly sandy loam C - 10 to 79 inches: stratified sand to gravel

#### Properties and qualities

Slope: 12 to 20 percent Depth to restrictive feature: More than 80 inches Drainage class: Excessively drained Runoff class: Low Capacity of the most limiting layer to transmit water (Ksat): High (2.00 to 6.00 in/hr) Depth to water table: More than 80 inches Frequency of flooding: None Frequency of ponding: None Calcium carbonate, maximum content: 45 percent Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm) Available water supply, 0 to 60 inches: Very low (about 2.6 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 6s Hydrologic Soil Group: A Ecological site: R108AY012IL - Outwash Prairie, R110XY018IL -Steep Gravel Prairie Forage suitability group: Low AWC, adequately drained with limitations (G095BY003WI) Other vegetative classification: Low AWC, adequately drained with limitations (G095BY003WI) Hydric soil rating: No



#### Minor Components

Fox

Percent of map unit: 10 percent Landform: Moraines Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope Down-slope shape: Linear Across-slope shape: Convex Hydric soil rating: No

### Data Source Information

Soil Survey Area: Jefferson County, Wisconsin Survey Area Data: Version 20, Sep 9, 2021

USDA	Natural Resources	Web Soil Survey	10/5/2021
-	Conservation Service	National Cooperative Soil Survey	Page 3 of 3



#### Figure 8: Wacousta Silty Clay Loam Data

Map Unit Description: Wacousta silty clay loam, 0 to 2 percent slopes----Jefferson County, Wisconsin Wacousta silty clay loam

#### Jefferson County, Wisconsin

#### Wa-Wacousta silty clay loam, 0 to 2 percent slopes

#### Map Unit Setting

National map unit symbol: 2tjx1 Elevation: 690 to 1,020 feet Mean annual precipitation: 32 to 35 inches Mean annual air temperature: 45 to 48 degrees F Frost-free period: 110 to 172 days Farmland classification: Prime farmland if drained

#### Map Unit Composition

Wacousta and similar soils: 85 percent Minor components: 15 percent Estimates are based on observations, descriptions, and transects of the mapunit.

#### **Description of Wacousta**

#### Setting

Landform: Interdrumlins Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Base slope Down-slope shape: Concave Across-slope shape: Concave Parent material: Stratified silty lacustrine deposits

#### Typical profile

Ap - 0 to 13 inches: silty clay loam Bg - 13 to 19 inches: silty clay loam Cg - 19 to 79 inches: silt loam

#### Properties and qualities

Slope: 0 to 2 percent Depth to restrictive feature: More than 80 inches Drainage class: Very poorly drained Runoff class: Negligible Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.60 to 2.00 in/hr) Depth to water table: About 0 inches Frequency of flooding: None Frequency of ponding: Frequent Calcium carbonate, maximum content: 30 percent Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm) Available water supply, 0 to 60 inches: Very high (about 12.6 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 3w

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-	Conservation Service

Map Unit Description: Wacousta silty clay loam, 0 to 2 percent slopes----Jefferson County, Wisconsin

Hydrologic Soil Group: B/D Forage suitability group: Frequently flooded, organics (G095BY010WI) Other vegetative classification: Frequently flooded, organics (G095BY010WI) Hydric soil rating: Yes

#### Minor Components

#### Sable

Percent of map unit: 8 percent Landform: Interdrumlins Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Base slope Down-slope shape: Concave Across-slope shape: Concave Hydric soil rating: Yes

#### Sebewa

Percent of map unit: 7 percent Landform: Interdrumlins Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Base slope Down-slope shape: Concave Across-slope shape: Concave Hydric soil rating: Yes

#### Data Source Information

Soil Survey Area: Jefferson County, Wisconsin Survey Area Data: Version 20, Sep 9, 2021



#### Figure 9: Boring 1

4	<b>3</b>	BEA OFES SER \	SIO /ICI	N A ES			LOG OF TEST BORING           roject         Lake Ripley Watershed Improvement           ocation         Cambridge, WI	Boring No.     1       Surface Elevation (ft)     844.0       Job No.     53775       Sheet     1_of								
	SA	MPL	E	-			VISUAL CLASSIFICATION	SOIL	SOIL PROPERTIES							
No.	G Rec	Moist	N	Des (f			and Remarks	qu (qa) (taf)	W	LL	PL	LI				
1	8	м	7	Ē	-		FILL: Gray/Brown Silty Fine to Coarse Sand and Lean Clay									
	•	TAX	-	14				(.75)	-	-	_					
2	6	м	2	1-1-				(.75)								
3	12	М	5	L-L-L			Medium Stiff to Stiff, Brown/Gray (Mottled) Lean CLAY (CL)	(1.25)	27.8							
4	18	М	3	LITT	10-			(0.5-0.75)	28.6							
5	12	W	17	rrd-trat	15-		Medium Dense, Brown Fine to Coarse SAND and GRAVEL, Some Silt (SM/GM) Occasional Thin, Soft Sandy Clay Seams	-								
6	6	w	15	LUJIL	20											
7	8	W	16	1414141	25-											
8	18	w	103		30	30-	30-	30-	30-		Very Dense, Yellow-White Fine to Medium SAND, Little Silt, Little to Some Gravel (SP-SM-Probable Weathered Sandstone Bedrock)	This be prepare Capston for edu	d for a e Engi	ŬW-	-Mad ing C	lison Class
				TTPJTT	35		End Boring at 30 ft Borehole backfilled with bentonite chips	does n conditio	ot repr	eser i sho r any	nt action	ual NOT				
	u		W	ATI	17.5		EVEL OBSERVATIONS	GENERA	LNC	TES	5					
Time Dept Dept	h to W h to Ci	Drillin ater ave in	-	_	_	-	Upon Completion of Drilling <u>NW</u> <u>15 Min</u> <u>2'</u> <u>24'</u> Driller Logger Drill Metho ay be gradual.	Chief Edito			Vig D-	120				

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#### Figure 10: Boring 2

4	*	BEA OFES SER \	SIO /ICI	ES		12	Cambridge, WI	Surface El Job No.	Boring No.         2           Surface Elevation (ft)         845.2           Job No.         53775           Sheet         1. of			
	SA	MPL	E			Γ	VISUAL CLASSIFICATION	SOIL	PRO	PER	TIE	s
No.	Rec (in.)	Moist	я		pth ft)		and Remarks	qu (qa) (taf)	W	LL	PL.	L
1	12	M	13	-			FILL: Dark Gray to Black Silty Fine to Coarse Sand					-
2	8	M	7	t-L						_		
-				F	5-	5	Soft to Medium Stiff, Gray-Green Lean CLAY (CL)	-	-	-	-	-
3	18	M	3	LIN				(.25-0.5)	34.2			_
4	12	W	5	11	10-		Medium Stiff to Stiff, Greenish Brown to Gray (Mottled) Lean CLAY, Trace to Little Gravel (CL)	(1.5)	25.4			-
				JILU								
5	12	W	14	11-1-	15			(0.75-1.0)	25.5			
6	10	w	15	LI THUTT	20-		Medium Dense, Brown Fine to Coarse SAND and GRAVEL, Some Silt (SM/GM)					
7	12	W	13	11111111	25-							
8	18	W	114	لما يلما	30-		Very Dense, Yellow-White Fine to Medium SAND, Little Silt, Little to Some Gravel (SP-SM-Probable Weathered Sandstone Bedrock)	This bo prepared Capstone for educ	d for a e Engi	ÚW- neer	Mad ing C	isor las
				Trhard-	35-		End Boring at 30 ft Borehole backfilled with bentonite chips	does no	ot repr	reser 1 sho 7 any	it acti uld N	ual IOT
			W	AT	EF	L		SENERA			5	-
Time Dept Dept	to W	Drillin ater ive in	g	3.0'	8, X0	_	Upon Completion of Drilling	Chief	HSA	_ F	lig D-	120

10wa-2



#### Figure 11: Boring 3

4	1	BEA OFES SER \	SIO VICI	ES		LOG OF TEST BORING Project Lake Ripley Watershed Improvement Location Cambridge, WI	Job No.	Boring No.         3           Surface Elevation (ft)         846.8           Job No.         53775           Sheet         1. of				
	SA	MPL	E	_	1	VISUAL CLASSIFICATION	SOIL	SOIL PROPERTI				
No.	T Rec P(in.)	Hoist	N	1.000	pth (t)	and Remarks	qu (qa) (taf)		IL	PL.	1.I	
1	12	м	6	LLL		FILL: Gray/Brown Silty Fine to Coarse Sand and Lean Clay		-	-	-	-	
2	18	М	9	TTT+	1		(2.0)	19.2		-	-	
3	18	М	4	Trlat	5-	Medium Stiff, Green-Gray to Black Lean CLAY	(0.5-1.0)	34.3	-		_	
4	18	M	8	FHIF		(CL) Stiff, Greenish Brown to Gray (Mottled) Lean CLAY, Trace to Little Gravel (CL)	(1.0)	16.3		-	-	
5	6	м	11	. Իսիդերիդերի	15-	Medium Dense, Brown Fine to Coarse SAND and GRAVEL, Some Silt (SM/GM) With Thin, Soft Sandy Clay Seams Near 15'						
6	8	w	21	rrd - rkal	20-							
7	6	w	15	Lulithon	25-			-				
8	18	w	100	LUJALU	30-	Very Dense, Yellow-White Fine to Medium SAND Little Silt, Little to Some Gravel (SP-SM-Probable Weathered Sandstone Bedrock)	prepare	This boring log has bee prepared for a UW-Madis Capstone Engineering Cl				
				TTP1TT	35-	End Boring at 30 ft Borehole backfilled with bentonite chips	does n conditio be us	for educational use only does not represent actu conditions and should N be used for any other purpose.			ual IOT	
					-	LEVEL OBSERVATIONS	GENERA	1224		5		
Time Depti Depti	to W	Drillin ater ave in	•		_	Upon Completion of Drilling <u>NW</u> <u>15 Min</u> <u>5'</u> <u>21.5'</u> Start 10, Driller Logger Drill Met	_ Chief	HSA	] 	Rig <u>D</u>	120	

lowa-2



#### Figure 12: Boring 4

4	3	BEA OFES SER \	SIO /ICI	N A			LOG OF TEST BORING roject Lake Ripley Watershed Improvement Cambridge, WI	Boring No. 4 Surface Elevation (ft) 843.8 Job No. 53775 Sheet 1 of 1					
	SA	MPL	E		1	-	VISUAL CLASSIFICATION	SOIL PROPERTIE				S	
No.	Rec (in.)	Moist	N		pth ft)	1	and Remarks	qu (qa) (tof)		LL	PL	LI	
1	8	M	7	5			FILL: Gray/Brown Silty Fine to Coarse Sand and Lean Clay						
				T				(.75)	-	-			
2	6	М	2	F				(.75)					
3	12	М	5	411			Medium Stiff to Stiff, Brown/Gray (Mottled) Lean CLAY (CL)	(1.25)	27.8				
4	18	М	3	41+1-	10-			(0.5-0.75)	28.6				
5	12	w	17	דרולדדלא	15-		Medium Dense, Brown Fine to Coarse SAND and GRAVEL, Some Silt (SM/GM) Occasional Thin, Soft Sandy Clay Seams						
6	6	W	15	լողերը	20-		10						
7	8	w	16	rrd-trbjr	25	· · · · · · · · · · ·							
8	18	w	103	بالليلال	30-		Very Dense, Yellow-White Fine to Medium SAND, Little Silt, Little to Some Gravel (SP-SM-Probable Weathered Sandstone Bedrock)	This boring log has been prepared for a UW-Madison Capstone Engineering Class for educational use only. It					
				LELLL	35-		End Boring at 30 ft Borehole backfilled with bentonite chips	does n conditio	ot repr	resen I sho r any	nt action	ual IOT	
//	u		W	AT	3. T. ( D - )	L	EVEL OBSERVATIONS	SENERA	LNC	TES	5		
Time Dept Dept	After h to W to Ca	Drillin ater ave in	-	-	_	_	Upon Completion of Drilling <u>NW</u> <u>15 Min</u> <u>2'</u> <u>24'</u> Oriller Driller Driller Drill Metho Drill Metho	_ Chief Editor	<b>HSA</b>	_ F	lig D-	120	

LOWA -2



#### Figure 13: Boring 5

4	4	BEA OFES SER \	SIC /ICI	NA ES		LOG OF TEST BORING Project Lake Ripley Watershed Improvement Location Cambridge, WI	Surface E Job No.	Boring No.         5           Surface Elevation (ft)         847.0           Job No.         53775           Sheet         1. of				
	SA	MPL	E			VISUAL CLASSIFICATION	SOIL	SOIL PROPERTIE				
No.	P Rec	Hoist	N		pth (t)	and Remarks	qu (qe) (taf)	w	IT	PL.	ы	
1	12	M	6	C L L		FILL: Gray/Brown Silty Fine to Coarse Sand and Lean Clay		-		-	-	
2	18	M	9	+41			(2.0)	19.2				
				T.	5-	Ħ	(2.0)	19.2	-	-		
3	18	M	4	111		Medium Stiff, Green-Gray to Black Lean CLAY (CL)	(0.5-1.0)	34.3				
4	18	М	8	L		Stiff, Greenish Brown to Gray (Mottled) Lean CLAY, Trace to Little Gravel (CL)	(1.0)	16.3			-	
				E	10-		(1.0)	10.5			-	
5	6	M	11	Trial		Medium Dense, Brown Fine to Coarse SAND and GRAVEL, Some Silt (SM/GM)	-	-				
				TLUT	15-	With Thin, Soft Sandy Clay Seams Near 15'		$\square$				
6	8	w	21	LAILL	20-			-				
				TTTT								
7	6	W	15	LUNT	25-							
8	18	w	100	-Luly		Very Dense, Yellow-White Fine to Medium SAND, Little Silt, Little to Some Gravel (SP-SM-Probable Weathered Sandstone Bedrock)	prepare	This boring log has bee prepared for a UW-Madi Capstone Engineering C for educational use only does not represent actu conditions and should N be used for any other purpose.			ison	
			<u>14*</u>	JTTPJTT	30-	End Boring at 30 ft Borehole backfilled with bentonite chips	for educed does n conditio				/. It ual IOT	
			W	4		LEVEL OBSERVATIONS	GENERA	LNC	TE	5		
Time Depti Dept	to W	Drillir ater ave in	g	18.0'	_	Upon Completion of Drilling <u>NW</u> <u>15 Min</u> <u>5'</u> <u>21.5'</u> Driller Logger Drill Methods to sont the approximate boundary between a may be gradual.	- Chief Edito	нг		Rig D	-120	

lowa-2



## LOG OF TEST BORING

General Notes

## Descriptive Soil Classification

#### GRAIN SIZE TERMINOLOGY

Soll Fraction

U.S. Standard Sleve Size

	Larger than 12"	Larger than 12"
*************	3" to 12"	3" to 12"
Coarse	3/4" to 3"	3/4" to 3"
Fine	4.76 mm to 3/4"	#4 to 3/4"
ozrse	2.00 mm to 4.76 mm	#10 to #4
Aodium	0.42 to mm to 2.00 mm	#40 to #10
Ino	0.074 mm to 0.42 mm	#200 to #40
*************	0.005 mm to 0.074 mm	Smaller than #200
	Smaller than 0.005 mm	Smaller than #200
	Coarse	Larger than 12"           3" to 12"           Coarse         3/4" to 3"           Fine         4.76 mm to 3/4"           coarse         2.00 mm to 4.76 mm           fedum         0.42 to mm to 2.00 mm           ins         0.074 mm to 0.47 mm           0.005 mm to 0.074 mm         0.005 mm           Smaller than 0.005 mm         5.001 mm

Particle Size

Plasticity characteristics differentiate between alt and day.

Tem

#### GENERAL TERMINOLOGY

RELATIVE DENSITY

Very Loose ..... 0-4

Loose ..... 4-10

Medum Dense ..... 10-30

Very Dense . ..... Over 50

"N" Valuo

Physical Characteristics	
Color, moisture, grain shape, fineness, etc.	
Major Constituents	
Clay, silt, sand, gravel	
Structure	
Laminated, varved, fibrous, stratified,	
cemented, fissured, etc.	
Geologic Origin	

Glacial, alkıvlal, eolian, residual, etc.

#### RELATIVE PROPORTIONS OF OF COHESIONLESS SOILS

Proportional Term	Defining Range by Percentage of Weight
Trace	
Little	12%-35%

#### ORGANIC CONTENT BY COMBUSTION METHOD

Soll Description	Loss on Ignition
Non Organic	Less than 4%
Organic Silt/Chry	
Sedmentary Peat	
Fibrous and Woody Peal	1 More than 50%

# Term q\_rtons/sq. ft. Very Soft 0.0 to 0.25 Soft 0.25 to 0.50 Medum 0.50 to 1.0 Stiff 1.0 to 2.0 Very Stiff 2.0 to 4.0 Hard Over 4.0

CONSISTENCY

#### PLASTICITY

Term	Plastic Index
None to Stight	0-4
Slight	
Medium	
High to Very High	Over 22

The penetration resistance, N, is the summation of the number of blows required to effect two successive 6" penetrations of the 2" split-barrel sampler. The sampler is driven with a 140 lb, weight failing 30" and is seated to a depth of 6" before commencing the standard penetration test.

#### SYMBOLS

#### DRILLING AND SAMPLING CS-Continuous Sampling

RC-Rock Coring: Size AW, BW, NW, 2"W RQD-Rock Quality Designator RB-Rock Bit FT-Fish Tell DC--Drove Casing C-Casing: Size 2 1/2", NW, 4", HW CW-Clear Water DM--DrEing Mud HSA-Hollow Stem Auger FA-Flight Auger HA-Hand Auger COA-Clean-Out Auger SS--2" Diameter Split-Barrel Sample 2ST--2" Diameter Thin-Walled Tube Sample 3ST--3" Diameter Thin-Walled Tube Sample PT-3" Diameter Piston Tube Sample AS-Auger Sample WS-Wash Sample PTS-Peat Sample PS--Pitcher Sample NR-No Recovery S-Sounding PMT-Borehole Pressuremeter Test VS-Vane Shear Test WPT-Water Pressure Test

#### LABORATORY TESTS

q.—Penetrometer Reading, tons/eq. ft. q.—Unconfined Strength, tons/eq. ft. W.-Molsture Centent, % LL.—Liquid Lintt, % PL—Plastic Linit, % SL—Shrinkage Linit, % LI—Loss on Ignition, % D—Dry Unit Weight, lbs/cu. ft. pH—Measure of Soil AlianInity or Actility FS—Free Swell, %

#### WATER LEVEL MEASUREMENT

▽ --Water Level at time shown NW--No Water Encountered WD--While Drilling BCR--Before Casing Removal ACR--After Casing Removal CW--Caved and Wet CM--Caved and Moist

Note: Water level measurements shown on the boring logs represent conditions at the time indicated and may not reflect static levels, especially in cohesive solls.



## UNIFIED SOIL CLASSIFICATION SYSTEM

## COARSE-GRAINED SOILS (More than half of material is larger than No. 200 seive size.)

Contraction of the local division of the loc		the many france of the stratesh
GRAVELS	GW	Well-graded gravels, gravel-sand mix- tures, little or no fines
More than half	GP	Poorty graded gravels, gravel-sand mix- tures, little or no floes
than No. 4	Gravels v	with Fines (Appreciable amount of fines)
SIEVE SIZE	·GM u d	Silty gravels, gravel-sand-silt mixtures
	GC	Clayey gravels, gravel-sand-clay mixtures
CIARSON AND	Clean Sar	nda (Little or no fines)
	SW	Well-graded sands, gravelly sands, little or no fines
SANDS More than half of coarse	SP	Poorly graded sands, graveliy sands, little or no fines
fraction smaller, than No. 4	Sanda wi	th Fines (Appreciable amount of fines)
sieve size	SMud	Silty sands, sand-silt mixtures

SC Clayey sands, sand-clay mixtures

## FINE-GRAINED SOILS

(More than half of material is smaller than No. 200 sieve.)

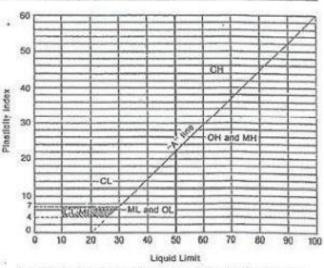
ML	Inorganic silts and very line sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity
CL	Inorganic clays of low to medium plastici- ty, gravelly clays, sandy clays, silty clays, lean clays
OL	Organic silts and organic silty clays of low plasticity
мн	Inorganic sills, micaceous or diatoma ceous fine sandy or silty soits, elastic silts
СН	Inorganic clays of high plasticity, fat clays
он	Organic clays of medium to high plasticity, organic silts
РТ	Peat and other highly organic soils
	CL OL MH CH OH

#### LABORATORY CLASSIFICATION CRITERIA

GW	$C_{cj} = \frac{D_{ab}}{D_{ab}} \text{ greater than 4; } C_{c} = -$	DuXDe between 1 and 3
GP	Not meeting all gradation req	ulrements for GW
GM	Atterberg limits below "A" tine or P.I. less than 4	Above "A" line with P.I. between 4 and 7 are
GC	Atterberg limits above "A" line with P.I. greater than 7	borderline cases requiring use of dual symbols
sw	$C_{ij} = \frac{D_{at}}{D_{st}}$ greater than 6; $C_{ij} =$	D <sub>10</sub> 2 <sup>2</sup> between 1 and 3
	the second s	
SP	Not meeting all gradation rec	ulrements for SW
SP SM	Not meeting all gradation no Atterberg limits below "A" line or P.I. tess than 4	Urements for SW Limits plotting in hatched zone with P.L. between 4 and 7 are borderine cases

PLASTICITY CHART





For classification of line-grained solls and fine fraction of coarsegrained solls.

Atterberg Limits ptotting in halched area are bordenline classifications requiring use of dual symbols. Equation of A-line: PI = 0.73 (LL - 20)

Lake Ripley Watershed 75% Geotechnical Report



#### **Document Qualifications and Limitations**

- This report is based on a unique set of project-specific factors. Even seemingly minor changes in the function, location, loading conditions or other factors assumed or provided to us for this report could affect the validity of the recommendations in this report. The geotechnical engineer should be notified of such changes and asked to review their impact on the recommendations.
- This report is based on the findings of the soil borings and test results performed for this project at the locations shown on the accompanying boring location plan. Variations in subsurface soil and groundwater conditions should be expected between boring locations. These variations or differences may not be apparent until construction. For this reason, your geotechnical engineer should be engaged to provide construction observation services so that appropriate modifications can be made, where necessary, in the recommendations in this report.
- Subsurface conditions including groundwater and soil conditions can change with time due to construction activities on this site or nearby properties, water table fluctuations, weather conditions and other factors. Construction observation services provided by your geotechnical engineer will help you address the impact of these changes on your project.
- Environmental concerns are NOT addressed in this report, as they were not included within the scope of our work. Professional consultation and exploration by a qualified environmental consulting firm is recommended where such concerns may exist.